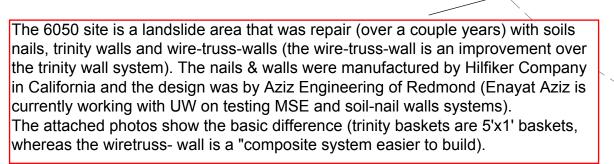
The 6050 site is a landslide area that was repair (over a couple years) with soils nails, trinity walls and wire-truss-walls (the wire-truss-wall is an improvement over the trinity wall system). The nails & walls were manufactured by Hilfiker Company in California and the design was by Aziz Engineering of Redmond (Enayat Aziz is currently working with UW on testing MSE and soil-nail walls systems). The attached photos show the basic difference (trinity baskets are 5'x1' baskets, whereas the wiretruss-wall is a "composite system easier to build).





L=12

L = 10

typical section HFM 6050 mainline road mp 2.7 slide repair

<u>_10</u>

18.6

57-

. *7*.5.

<u>=</u>10

3.8

4.5

3.0

14.0

3.5

3.0

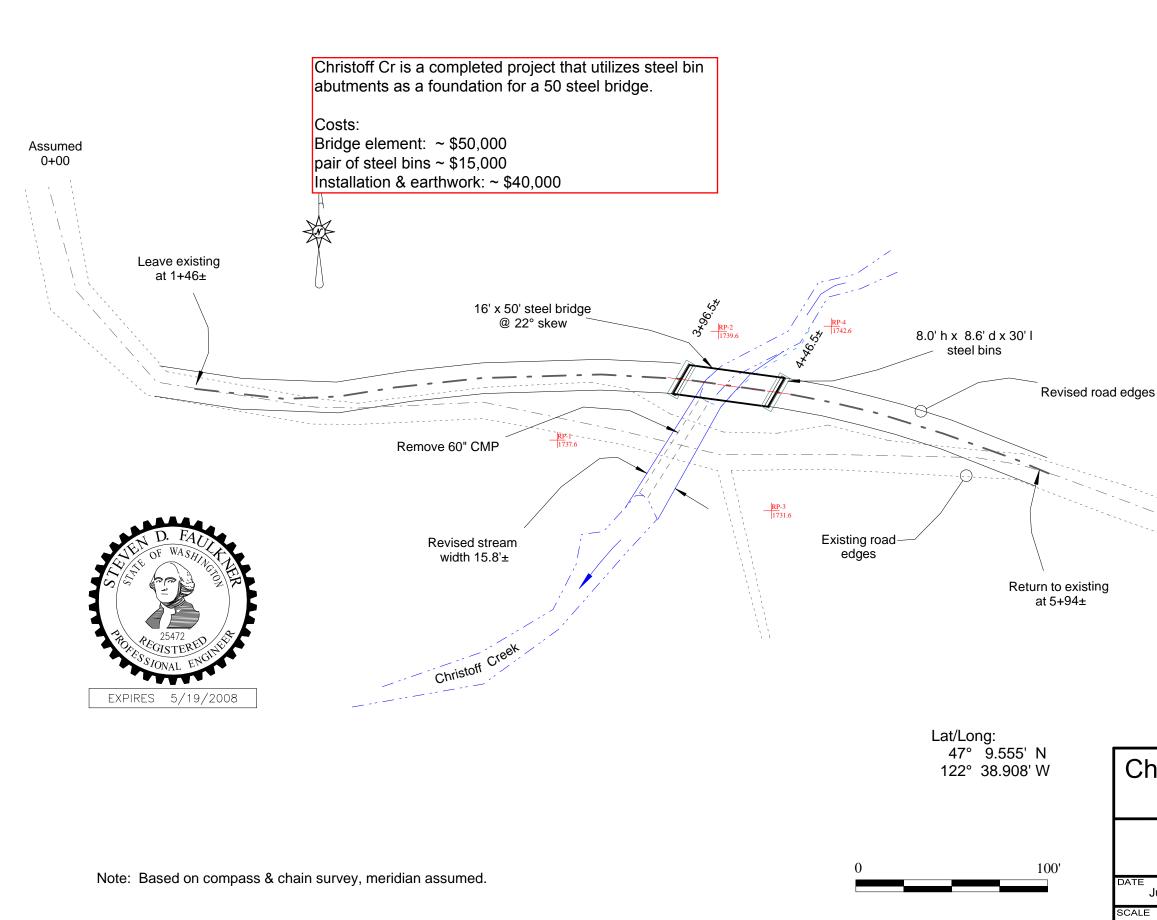
scale 1"= 5ft vert & horiz









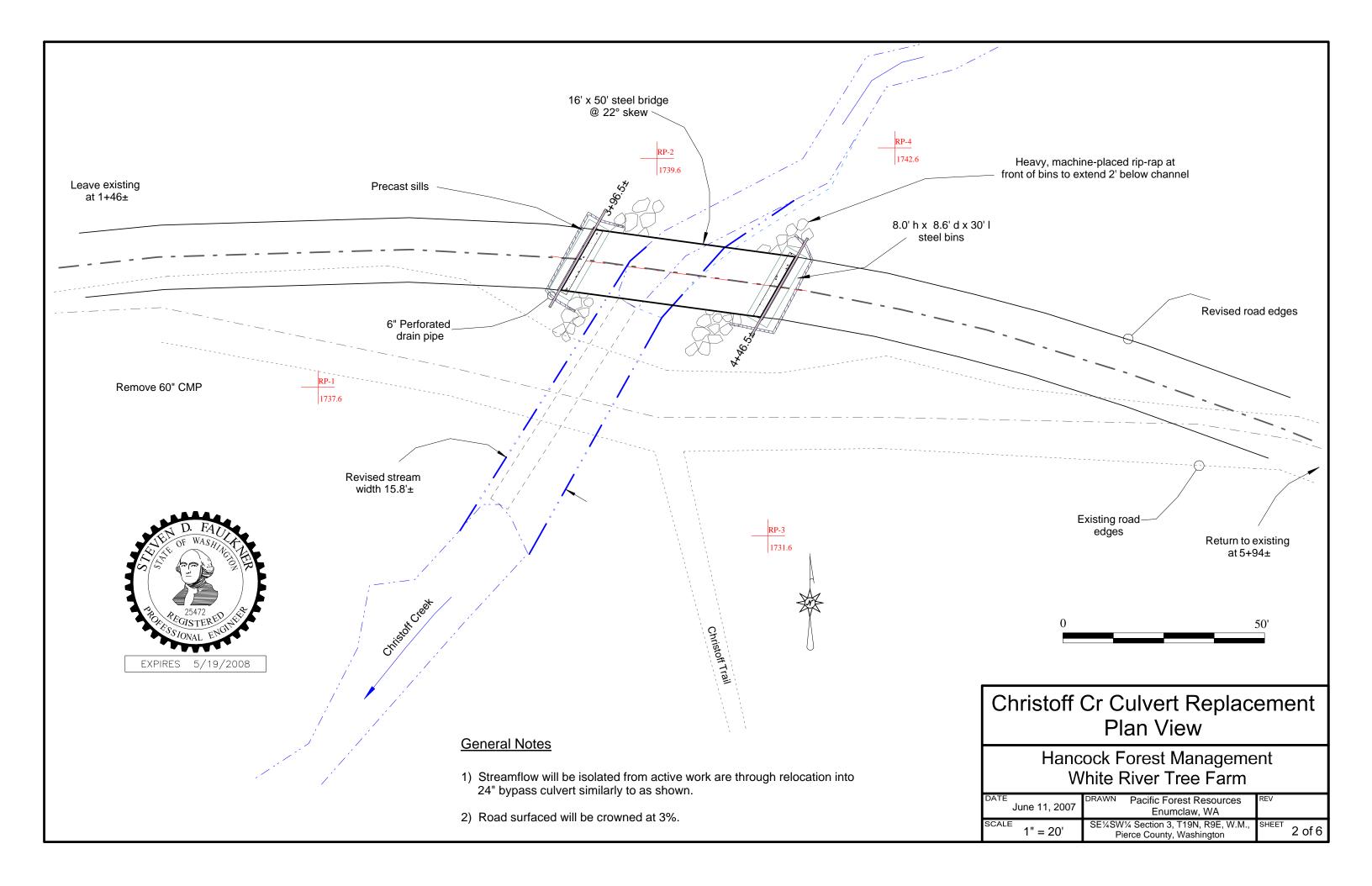


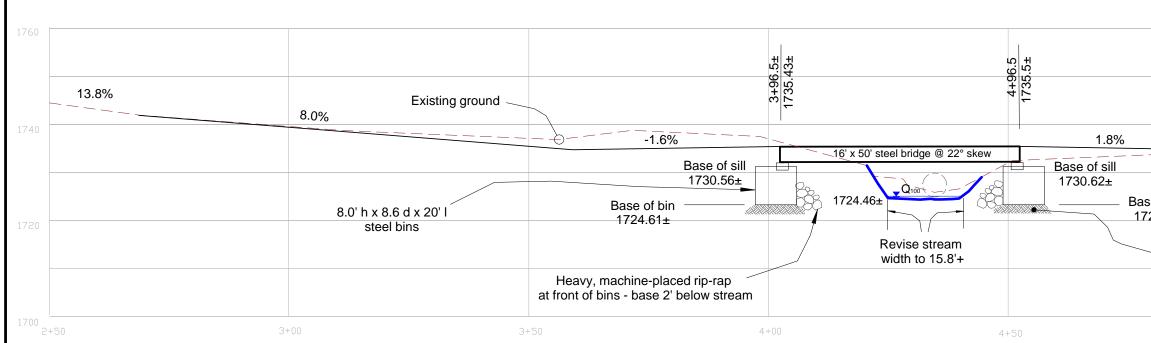
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| Site Overview | 1 |
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| Stake-out Data | 6 |
| | |

Christoff Cr Culvert Replacement Site Overview

| June 11, 2007 | DRAWN Pacific Forest Resources Enumclaw, WA | REV |
|-------------------------|---|-------------------------|
| . ^E 1" = 50' | SE¼SW¼ Section 3, T19N, R9E, W.M., Pierce County, Washington | ^{SHEET} 1 of 6 |

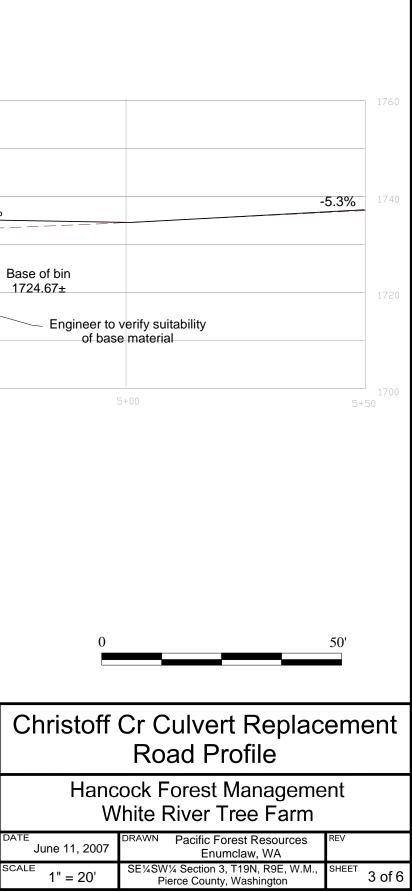






General Notes

- 1) Bin footing shall be sub-excavated, backfilled and compacted with suitable material as directed by Design Engineer.
- 2) Bin backfill material to be well-graded, 3-inch minus crushed material. Backfill material to be approved by engineer prior to placement.
- 3) Streambed material as approved by WDFW biologist will be placed within revised channel.
- 4) Elevation interpolated from GPS. Vertical and horizontal data developed from compass and chain survey.

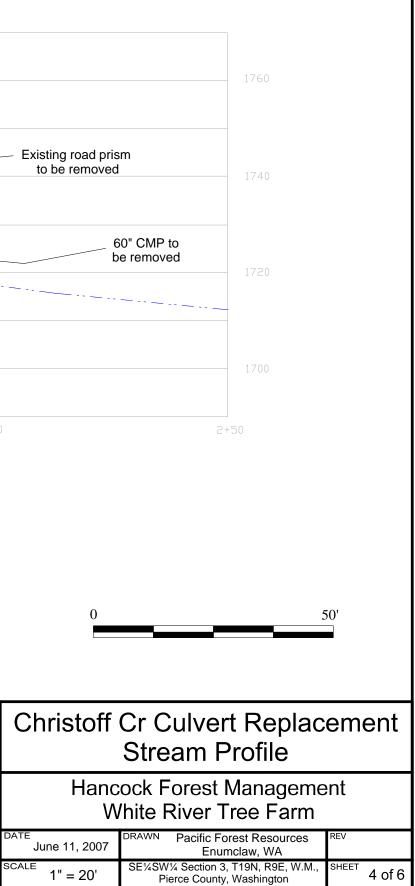


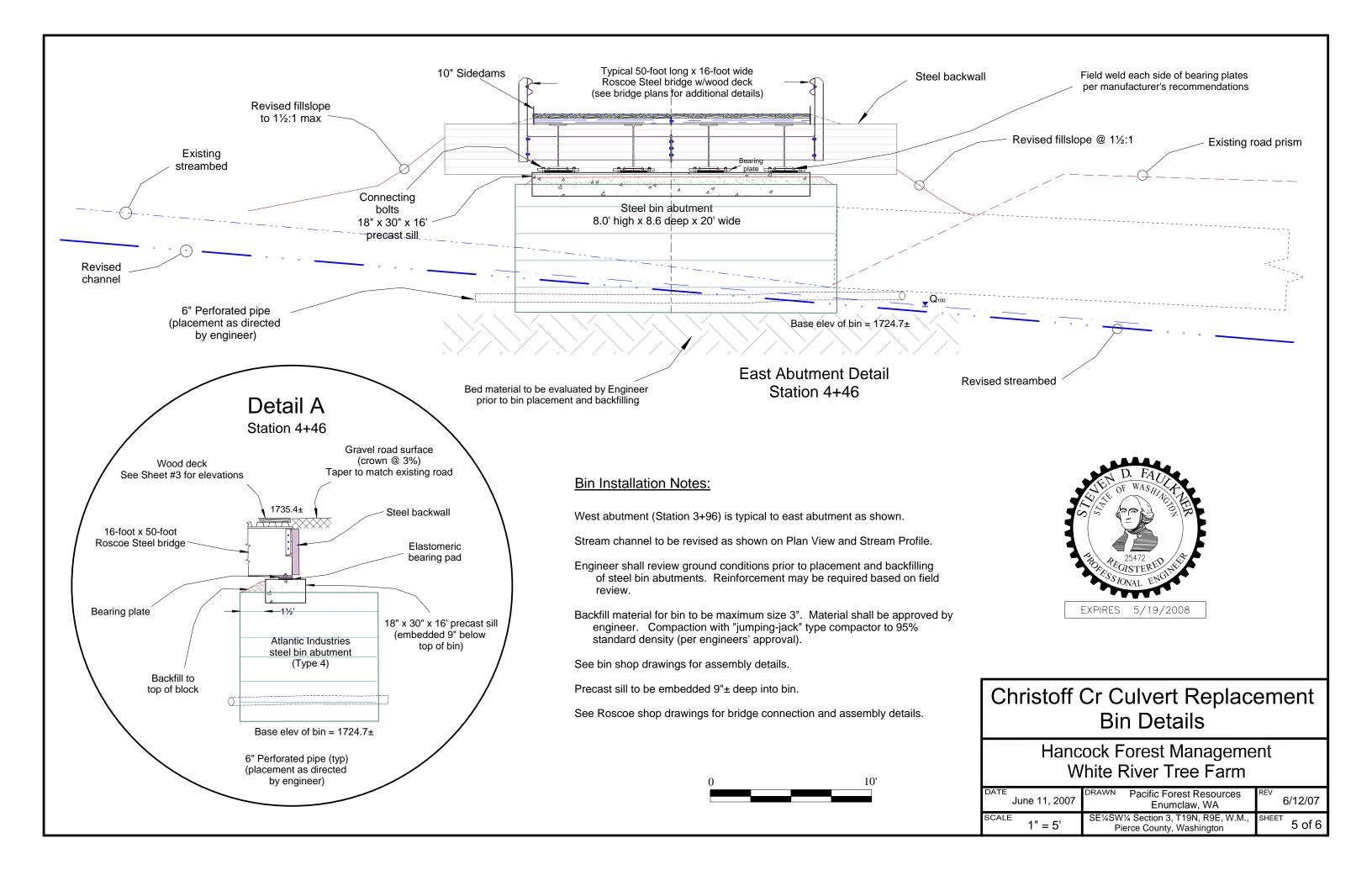


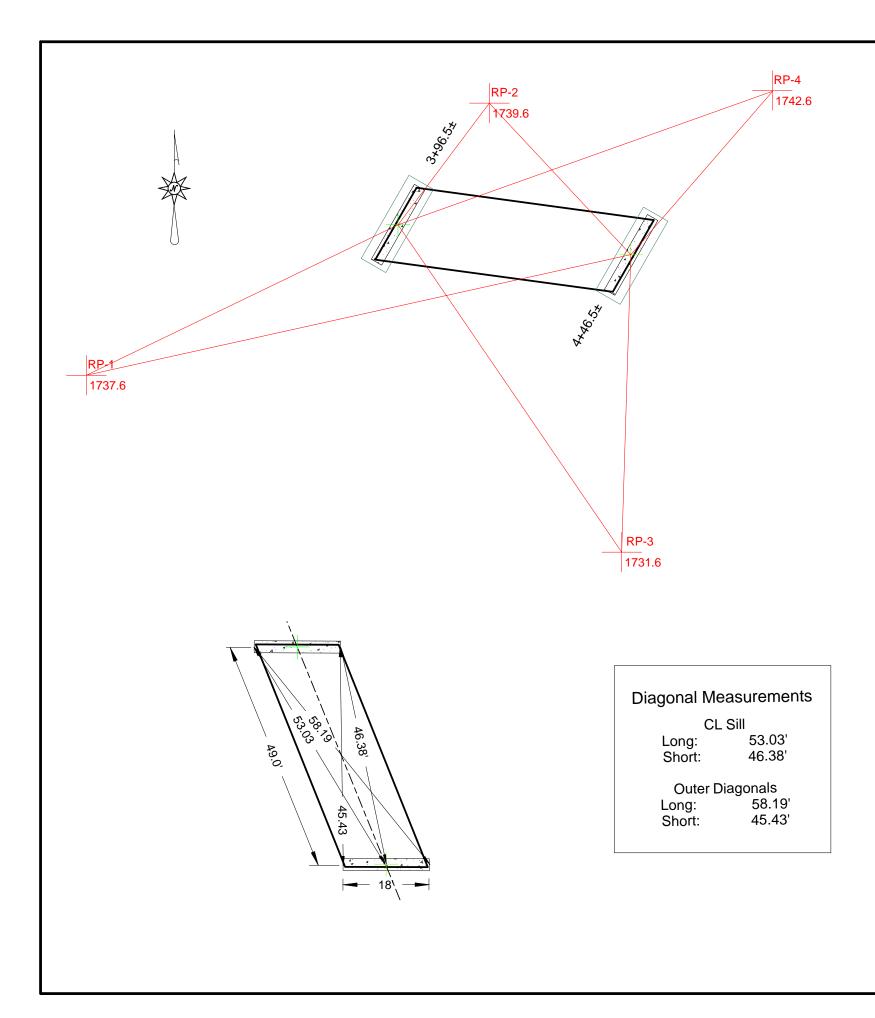


General Notes

- 1) Streambed material as approved by WDFW biologist will be placed within revised channel.
- 2) Any rip-rap placed adjacent to streambanks must be able to withstand 100-year flows (2-man+ rocks).

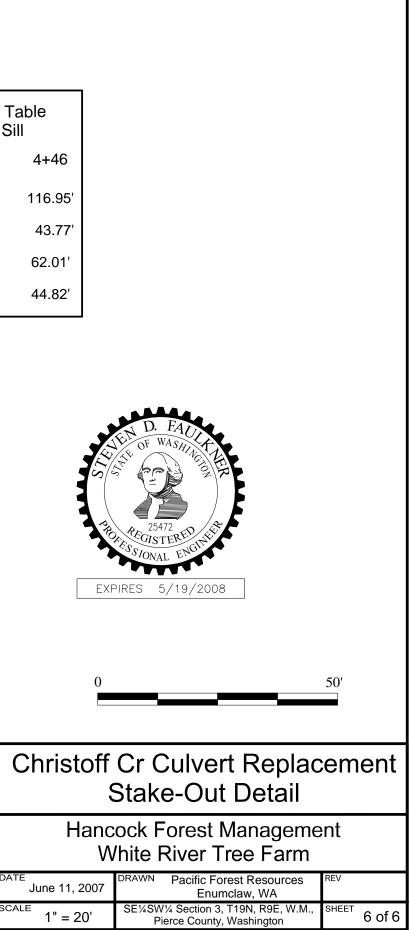






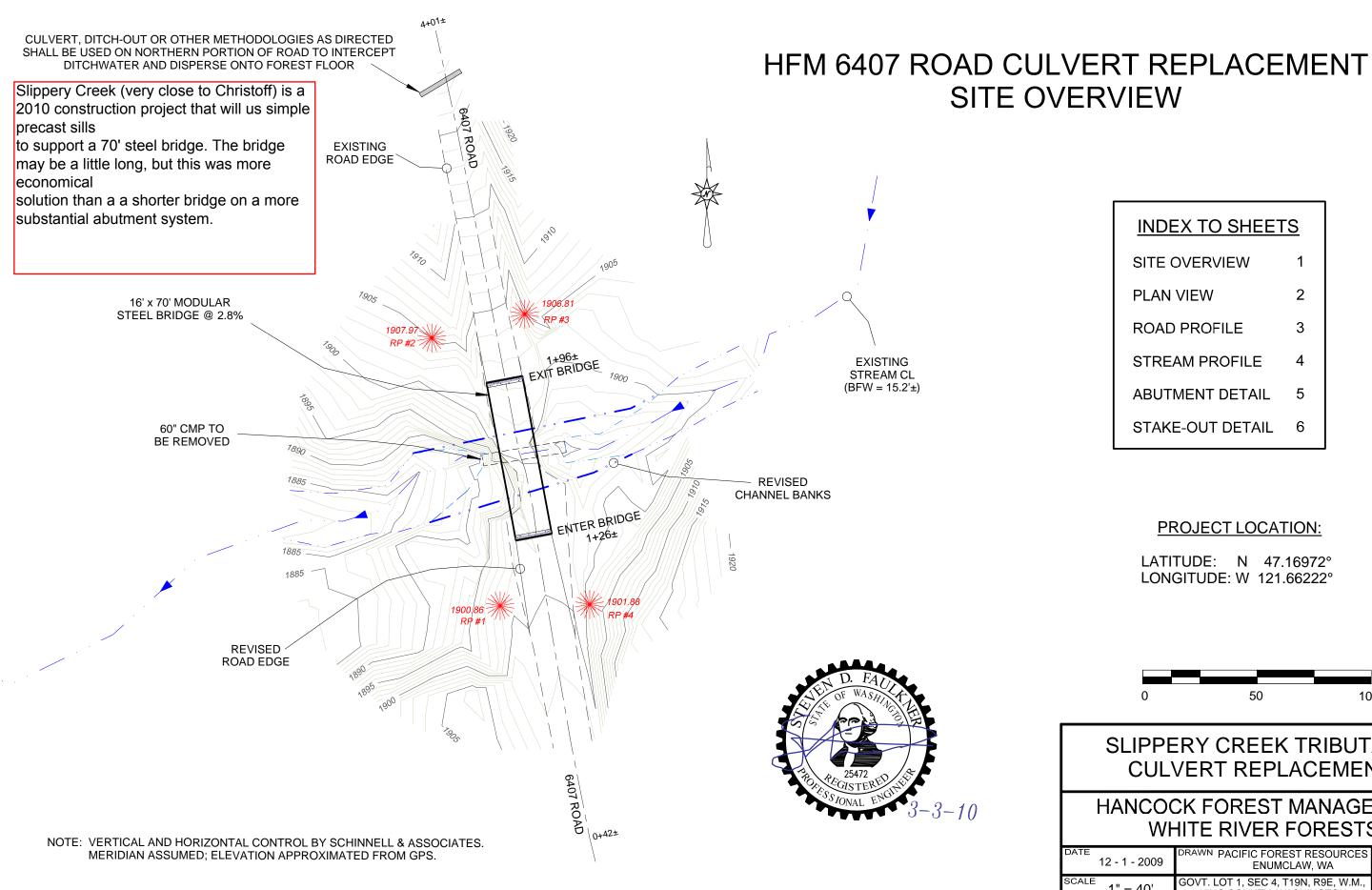
| S | itake-Out Tab to CL of Sill |
|-------|--------------------------------|
| Point | 3+96 |
| RP-1 | 72.36' |
| RP-2 | 31.53' |
| RP-3 | 82.76' |
| RP-4 | 82.79' |

SCALE





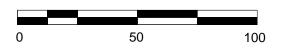




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| PLAN VIEW | 2 |
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| STREAM PROFILE | 4 |
| ABUTMENT DETAIL | 5 |
| STAKE-OUT DETAIL | 6 |



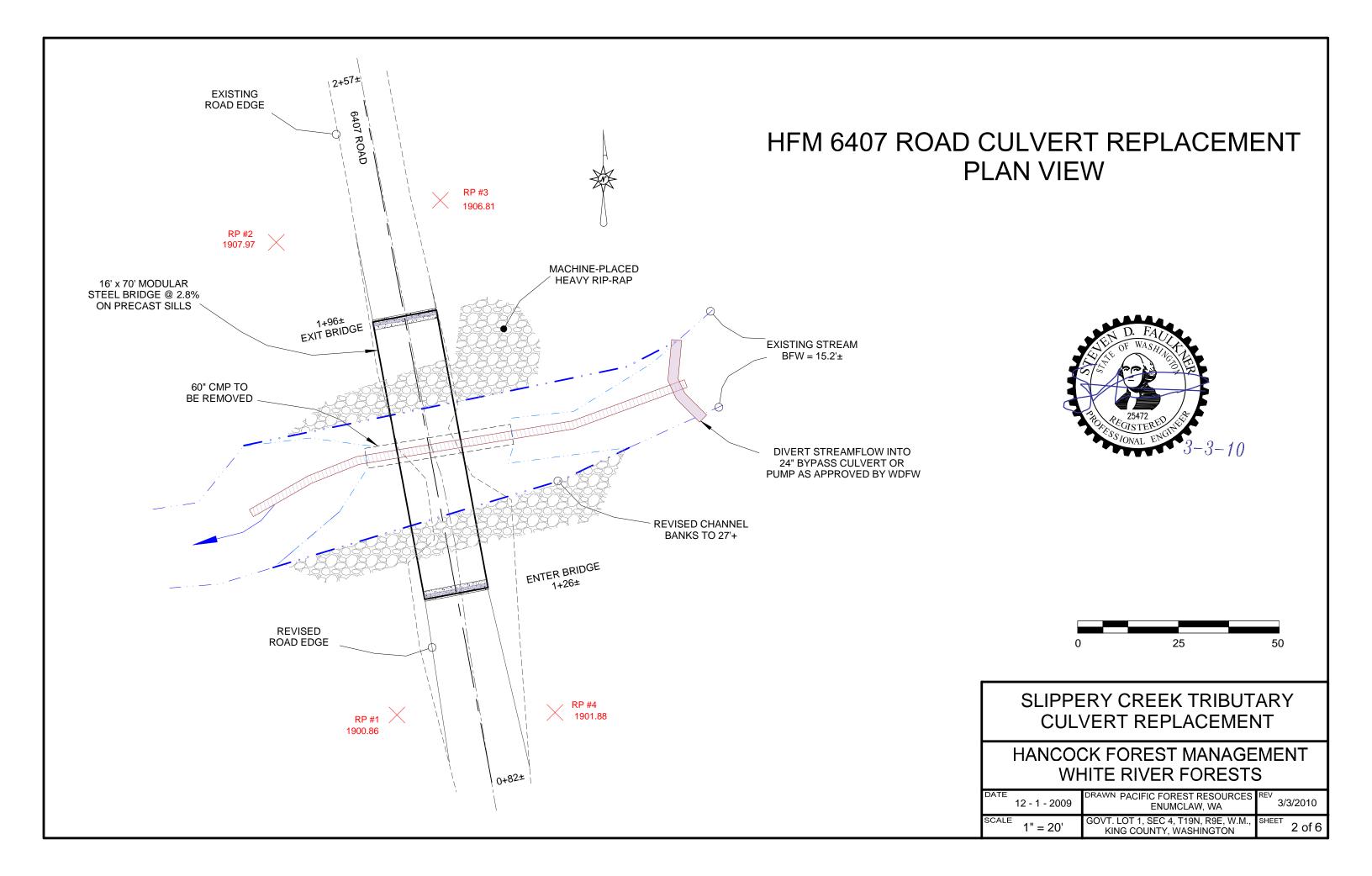
LATITUDE: N 47.16972° LONGITUDE: W 121.66222°



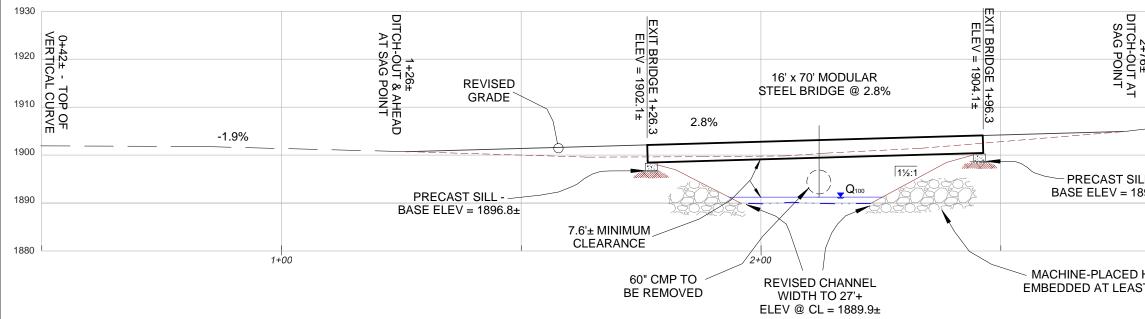
SLIPPERY CREEK TRIBUTARY CULVERT REPLACEMENT

HANCOCK FOREST MANAGEMENT WHITE RIVER FORESTS

| 12 - 1 - 2009 | DRAWN PACIFIC FOREST RESOURCES ENUMCLAW, WA | REV 3/ | /3/2010 |
|-----------------------|---|-----------|---------|
| ^E 1" = 40' | GOVT. LOT 1, SEC 4, T19N, R9E, W.M., KING COUNTY, WASHINGTON | SHEET | 1 of 6 |

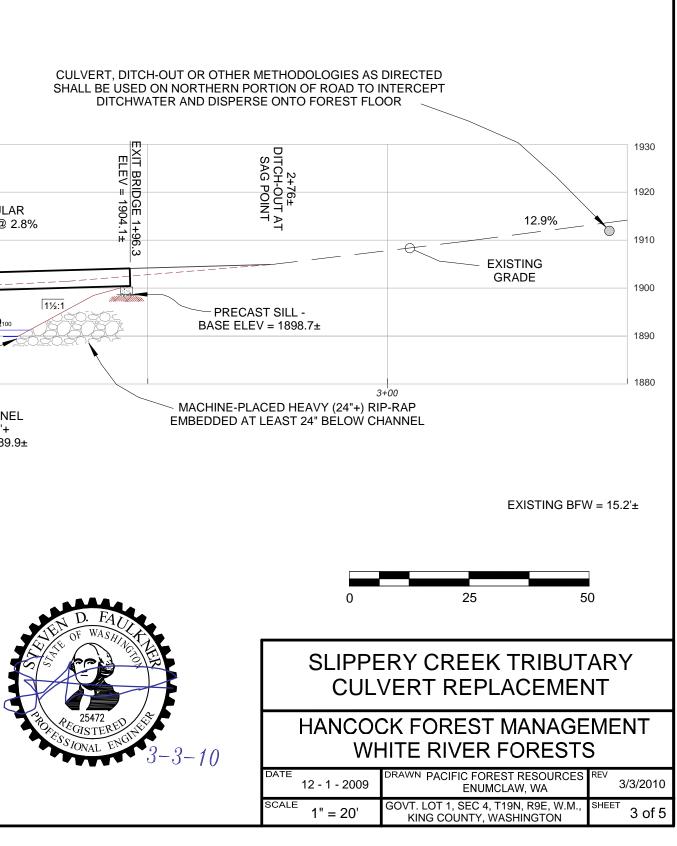


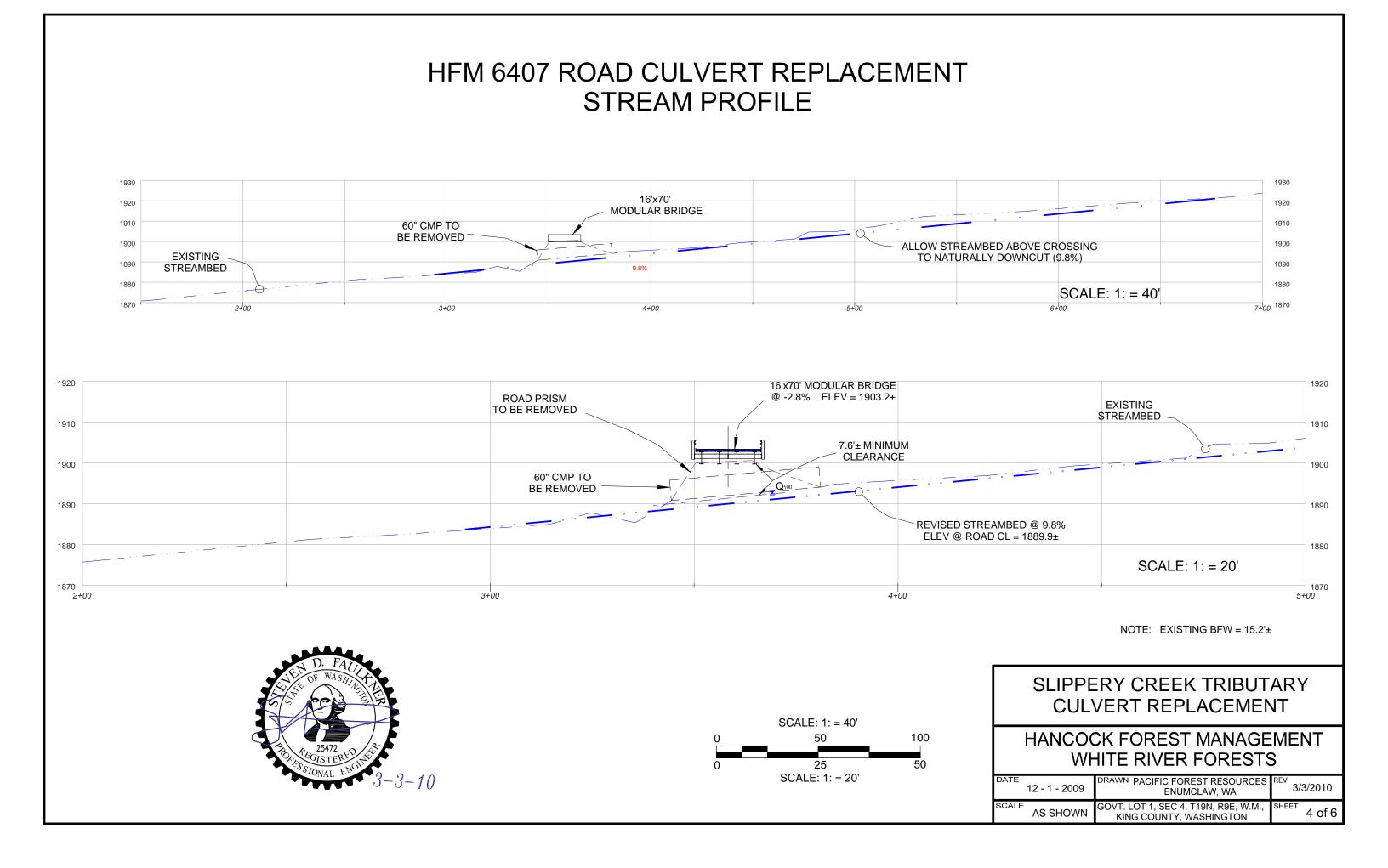
HFM 6407 ROAD CULVERT REPLACEMENT **ROAD PROFILE**

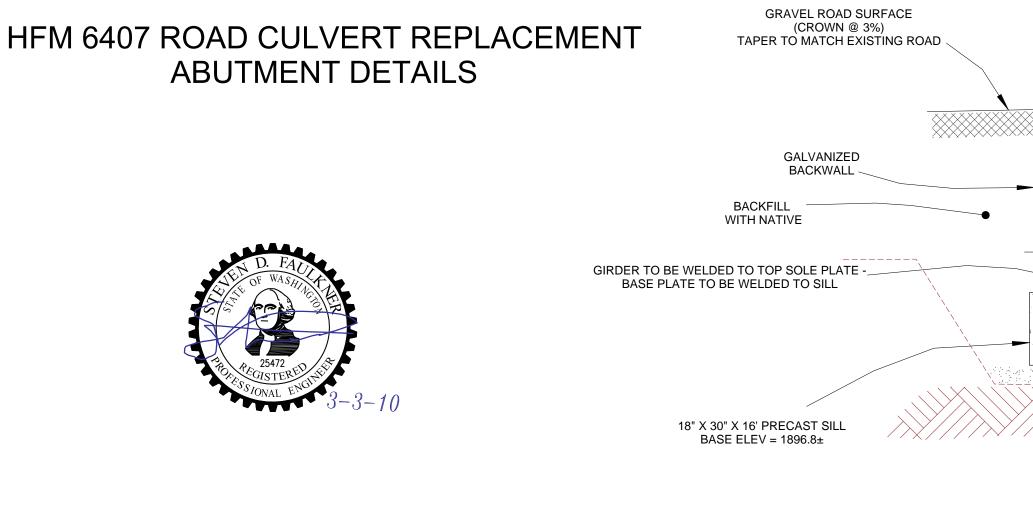


GENERAL CONSTRUCTION NOTES

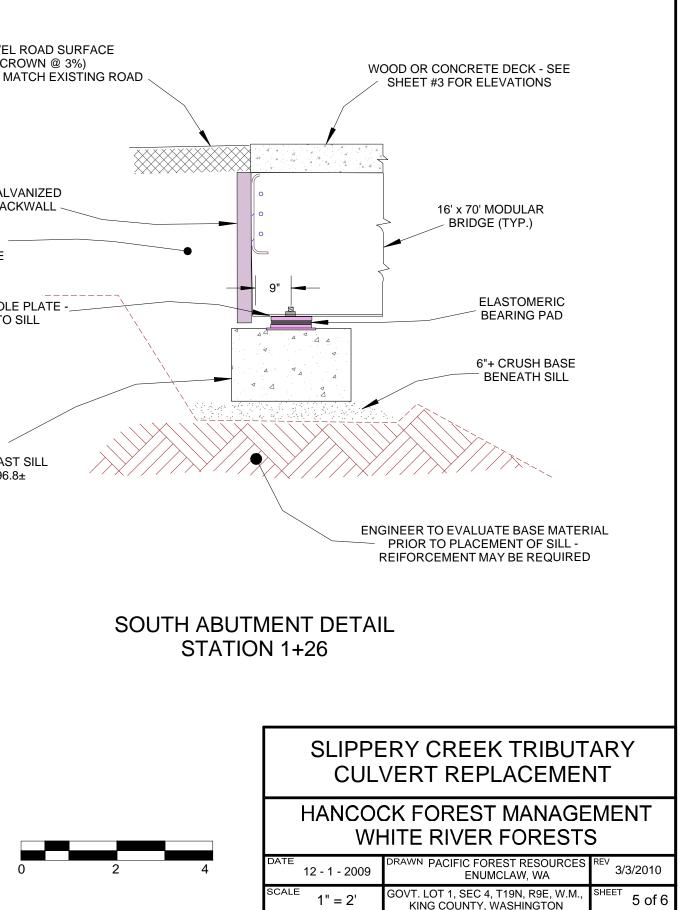
- 1) 16' x 70' STRUCTURE IS A MODULAR STEEL BRIDGE WITH A WOOD OR CONCRETE DECK: LOAD RATING IS U-80.
- 2) PRECAST SILLS SHALL BE SET ON AT LEAST 6" OF CRUSHED MATERIAL (2" MINUS OR SMALLER); MAXIMUM SLOPE FROM TOE OF SILL TO CHANNEL SHALL BE 2:1 MAXIMUM. SOIL CONDITIONS AT BASE OF SILLS TO BE EVALUATED BY ENGINEER PRIOR TO PLACEMENT OF SILLS: BASE REINFORCEMENT MAY BE REQUIRED PER ENGINEER'S EVALUATION.
- 3) RIP-RAP TO BE MINIMUM 24" MATERIAL AND SHALL BE FOUNDED AT LEAST 24-INCHES BELOW CHANNEL BED.
- 4) STREAMFLOW TO BE DIVERTED FROM WORK AREA APPROXIMATELY AS SHOWN ON PLAN VIEW OR AS OTHERWISE APPROVED BY HPA.
- 5) A CULVERT, DITCH-OUT OR OTHER METHODOLOGIES AS DIRECTED SHALL BE USED ON NORTHERN PORTION OF ROAD TO INTERCEPT DITCHWATER AND DISPERSE ONTO FOREST FLOOR. EXACT PLACEMENT SHALL BE FIELD LOCATED DURING BRIDGE CONSTRUCTION OPERATIONS
- 6) ALL EXPOSED SOILS TO BE STRAW MULCHED AND SEEDED.
- 7) ROAD SURFACED WILL BE CROWNED AT 3%.

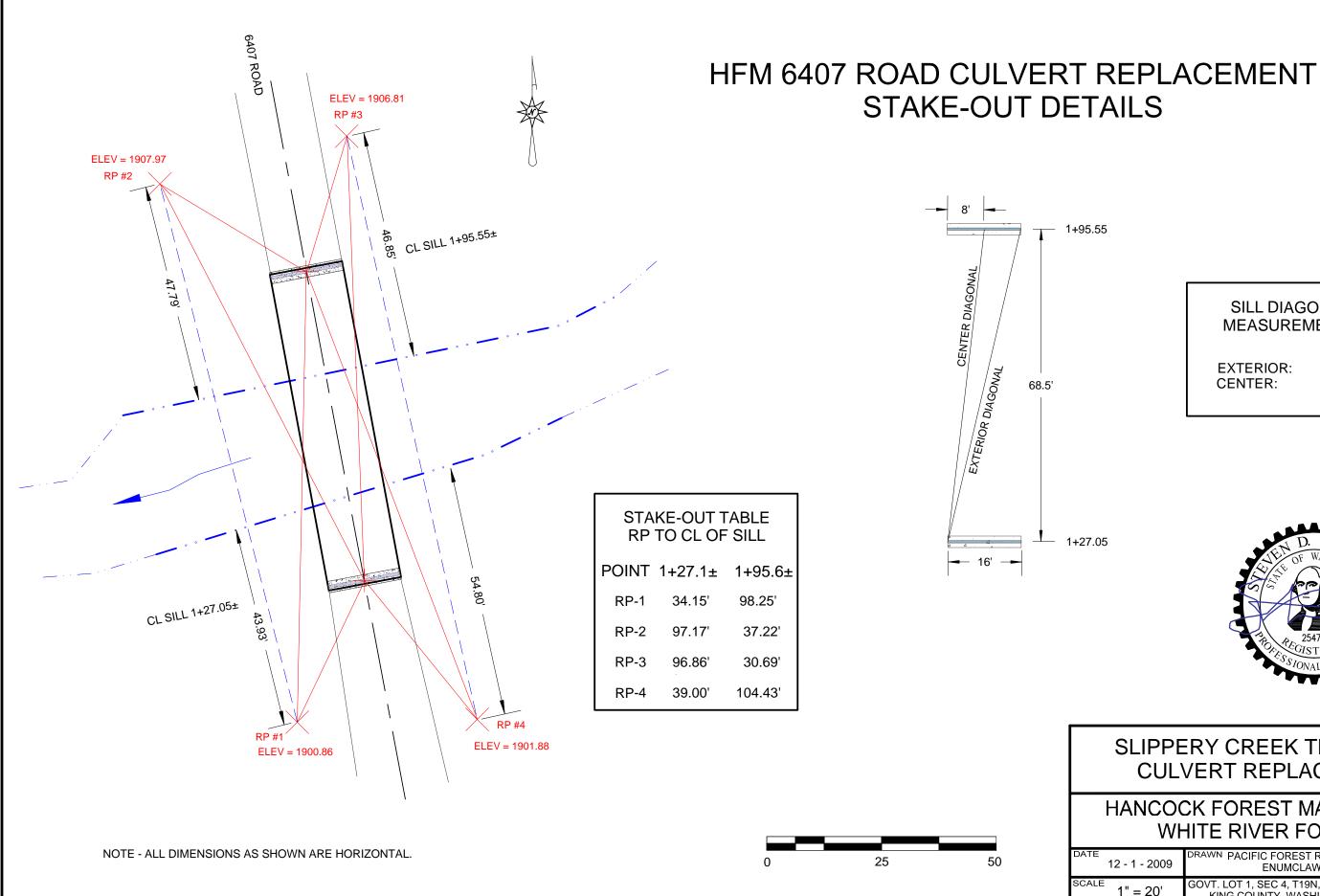






- 1) NORTH ABUTMENT (STATION 1+96) IS TYPICAL TO SOUTH ABUTMENT AS SHOWN (ELEV @ BASE OF 1+96 SILL = 1898.7±)
- 2) STREAM CHANNEL TO BE REVISED TO AT LEAST 27-FEET WIDE AS SHOWN ON PLAN AND PROFILE VIEWS.
- 3) AT LEAST 6-INCHES OF CRUSHED BASE MATERIAL SHALL BE PLACE BENEATH SILL AS LEVELING COURSE.
- 4) ENGINEER SHALL REVIEW GROUND CONDITIONS PRIOR TO PLACEMENT PRECAST SILL ABUTMENTS; REINFORCEMENT MAY BE REQUIRED BASED ON FIELD REVIEW.
- 5) REFER TO MANFACTURER'S SHOP DRAWINGS FOR BRIDGE ASSEMBLY AND CONNECTION DETAILS.

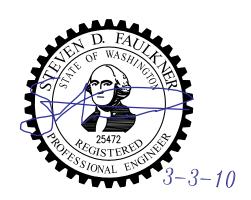




SILL DIAGONAL **MEASUREMENTS**

EXTERIOR: CENTER:

70.34' 68.97'



SLIPPERY CREEK TRIBUTARY CULVERT REPLACEMENT

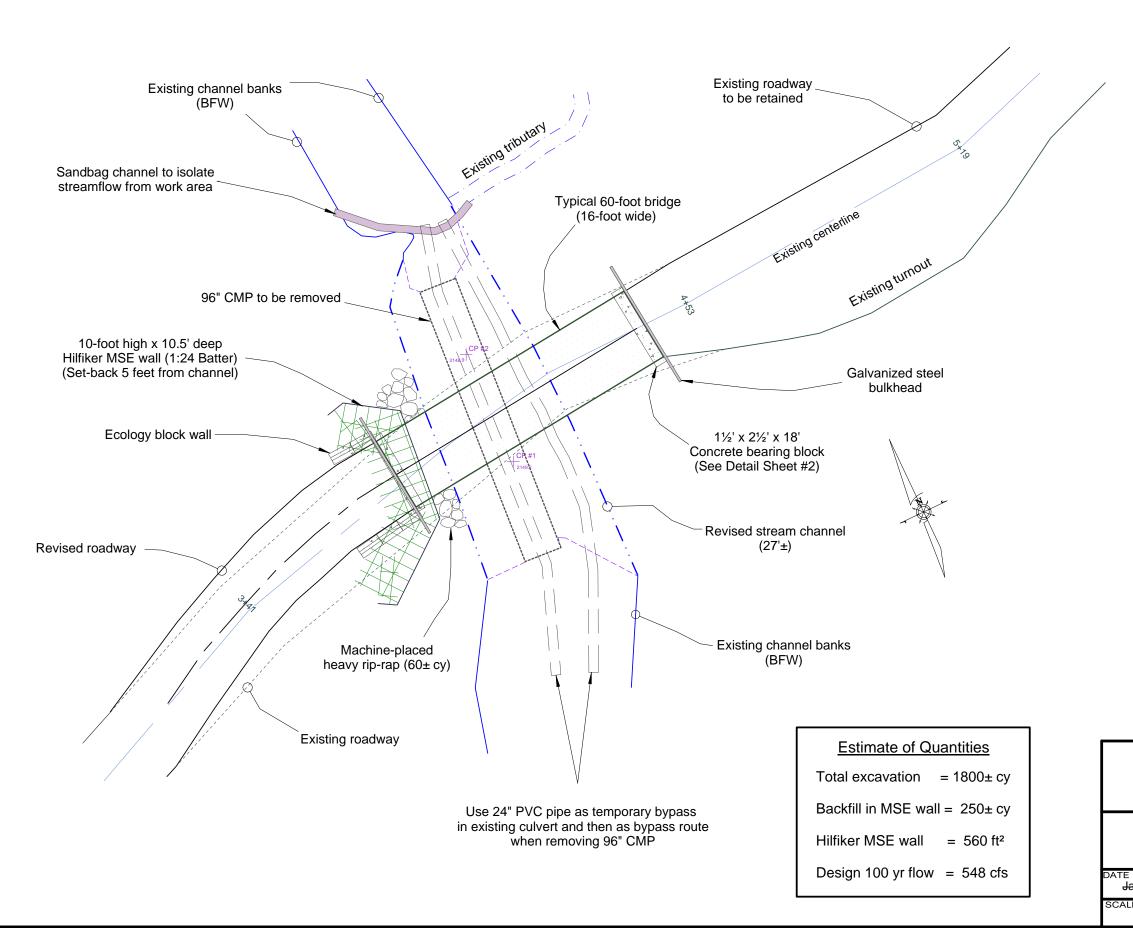
HANCOCK FOREST MANAGEMENT WHITE RIVER FORESTS

| 12 - 1 - 2009 | DRAWN PACIFIC FOREST RESOURCES ENUMCLAW, WA | REV 3/3/2010 |
|-----------------------|---|-------------------------|
| ^E 1" = 20' | GOVT. LOT 1, SEC 4, T19N, R9E, W.M., KING COUNTY, WASHINGTON | ^{SHEET} 6 of 6 |

Scatter Creek Bridge

Scatter Cr (or 5200 Bridge) is a completed project utilizing a 60' steel bridge supported on one side by a simple precast sill and rip-rap and on the other by a complex MSE wall system.





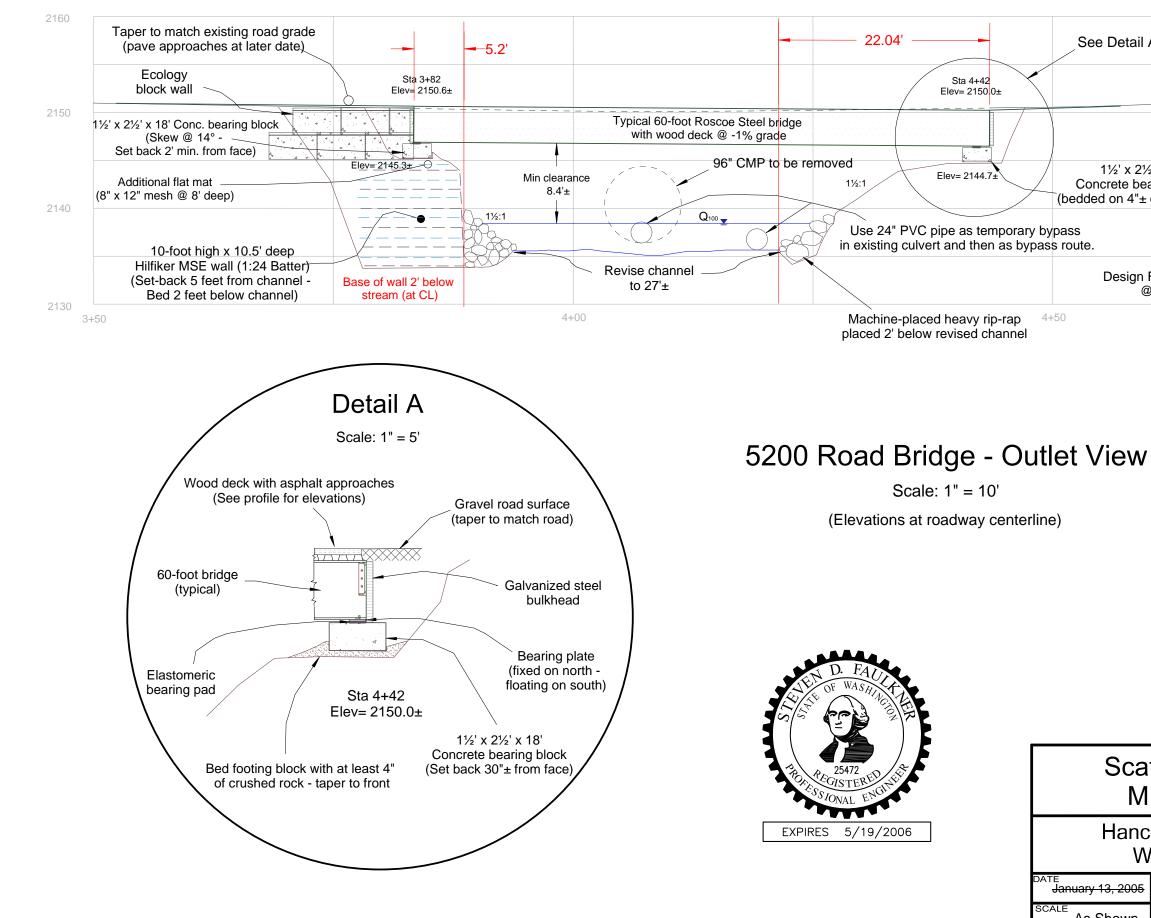
Index to Sheets

- Plan View1Road Profile2Stream Profile3
- MSE Wall Layout 4



Scatter Creek Bridge Site M.P. 3.7 - 5200 Road

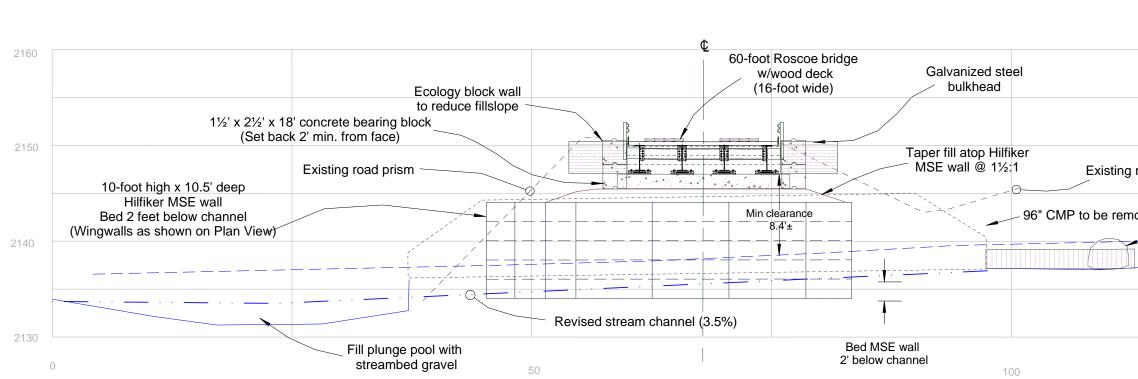
| anuary 13, 2005 | DRAWN Pacific Forest Resources Enumclaw, WA | REV 3/3/2005 |
|-----------------|--|--------------|
| _E 1" = 20' | NE¼NE¼ Section 25, T20N, R7E, W.M., King County, Washington | SHEET 1 of 4 |



| See Detail A |
|--|
| |
| |
| 1½' x 2½' x 18' Concrete bearing block bedded on 4"± crushed rock) |
| ass route. |
| Design Flow (Q₁∞) = 548 cfs @ 2141± acres |
|) |

| Scatter Creek Bridge Site |
|---------------------------|
| M.P. 3.7 - 5200 Road |

| anuary 13, 2005 | DRAWN Pacific Forest Resources Enumclaw, WA | ^{REV} 3/3/2005 |
|-----------------|---|-------------------------|
| E As Shown | NE ¹ / ₄ NE ¹ / ₄ Section 25, T20N, R7E, W.M., King County, Washington | SHEET 2 of 4 |



Notes:

Proposed bridge is 60-foot Roscoe Steel structure with U-80 load rating.

- Hilfiker MSE wall is 1:24 batter; 10' high x 10.5' deep. W-7.0 wire to be used with intermediate mats placed at 1' spacing. Additional flat mat (8" x 12" mesh) to be placed 8" above standard top mat, with precast sill place above flat mat as shown. Design life is 75 years.
- Precast sills to be 18" high x 30" wide x 18' long; concrete to be 5000 psi. Sill to be bedded on at least 4" of crushed rock. Assumed soil bearing capacity 4000+ psf.
- Stream channel to be reshaped as shown; retain existing CMP until bypass is complete. Streamflow is to be isolated from work area via bypass culverts or pumping as shown.

Bridge deck to be wood, taper approaches to match existing road grade.

See bridge shop drawings for bridge fabrication and connection details.

"Fixed" bearing plate to be installed on northern abutment, "floating" bearing plate on southern abutment.

5200 Road Bridge - South Abutment View Station 3+82

(Elevations at stream centerline)



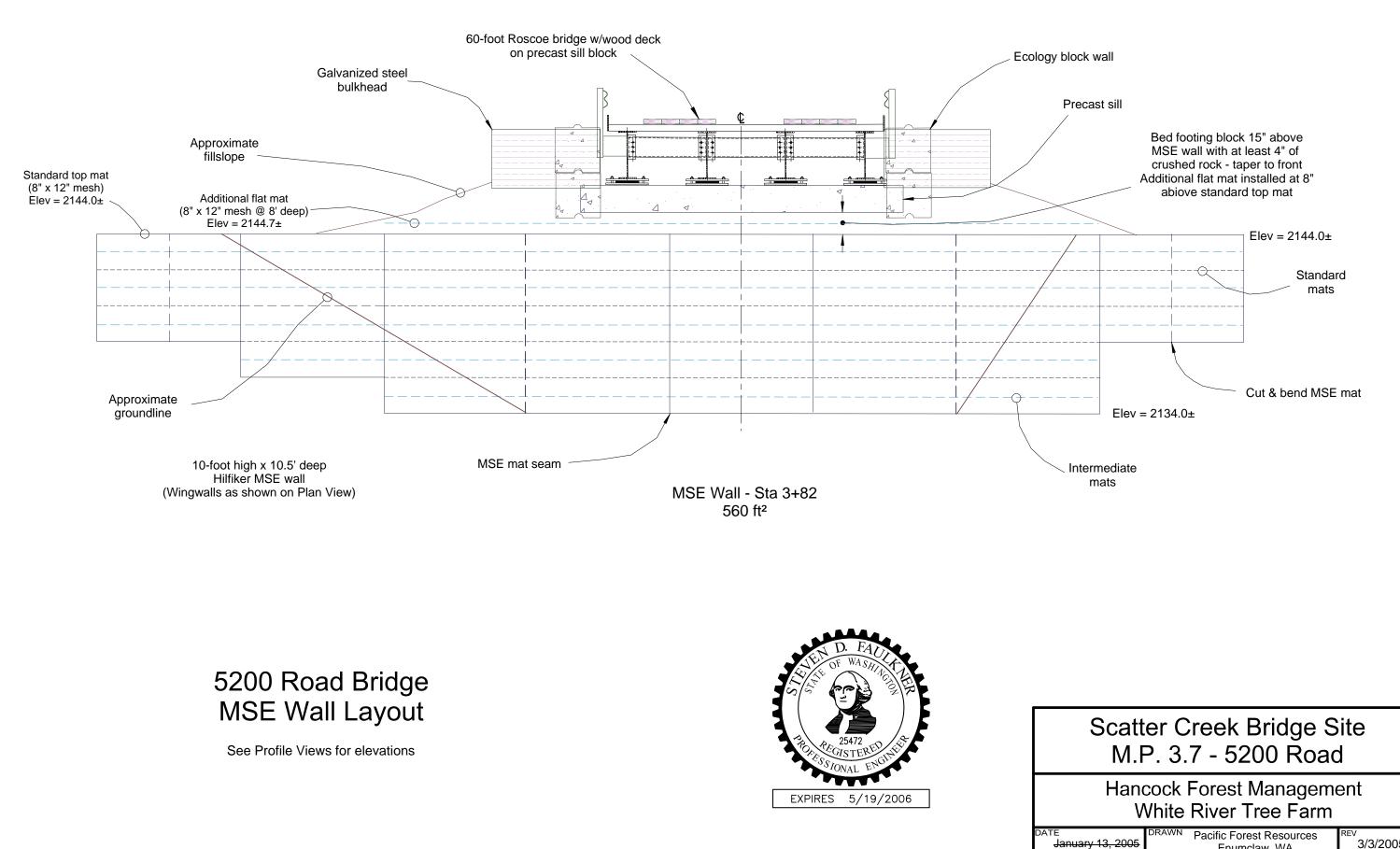


| | andbag channel & bypass into temporary |
|-------|--|
| red p | ipe to isolate streamflow from work area |
| | Q ₁₀₀ (E = 2138.4± @ CL) |
| | Existing stream channel |
| | |
| | |

150

Scatter Creek Bridge Site M.P. 3.7 - 5200 Road

| anuary 13, 2005 | DRAWN Pacific Forest Resources Enumclaw, WA | REV 3/3/2005 |
|-----------------|--|-------------------------|
| E 1" = 10' | NE¼NE¼ Section 25, T20N, R7E, W.M., King County, Washington | ^{SHEET} 3 of 4 |



| DATE January 13, 2005 | | ^{REV} 3/3/2005 |
|--------------------------|---|-------------------------|
| scale 1" = 5' | NE ¹ / ₄ NE ¹ / ₄ Section 25, T20N, R7E, W.M., King County, Washington | SHEET 4 of 4 |

Hilfiker MSE Wall Design Notes

1. Wall design is based on construction methods and materials conforming with the requirements of the Hilfiker Retaining Walls. The Hilfiker welded wire wall 24"-lift construction guide, together with the specifications for this project are an integral part of these drawings. Selected material for this project is welded w7.0 galvanized wire with 8" wide by 21" long transverse spacing. Mid-layer flat mats are required as shown on the attached plans.

2. Except for the facing zone (see details), wall fill or backfill shall be in conformance with the Hilfiker Retaining Walls specifications. Fill for the MSE wall up to minimum elevations as shown (i.e., below Q₁₀₀ elevation), shall be 4" to 6" angular quarry spalls (sound hard rock, minimum 2 fractured sides 90% of particle composition). Above that elevation, the MSE wall may be backfilled with native material per engineer's acceptance. The minimum fill and backfill compaction shall be 95% of the maximum density. Compaction shall be determined in accordance with compaction test results and design engineer's review and approval. Drainage control shall be as directed by the engineer in the field.

3. Compaction within 3 feet of the face of wall shall be accomplished by at least 3 passes of a light-weight mechanical tamper, roller or vibratory system per engineer's direction. The facing zone may be filled with 2" minus crushed aggregate (minimum 2 fractured sides 90% of particle composition) densified by rodding using a heavy steel rod. Compaction tests will not be necessary within 3 feet of the face of wall.

4. Wall design parameters are estimated based on site observations. Existing ground shown is based on field survey. Minor revisions to this plan may be required by the design engineer following inspection of site during construction operations. Hilfiker Retaining Walls' responsibility is for the material only, based on the assumption of design parameters outlined below.

5. Unsuitable foundation soils below the wall base level, as determined by the engineer, shall be overexcavated and replaced with granular material compacted to 95% of maximum density as directed by the engineer.

6. Geostructural design parameters - wall backfill: compacted unit weight 125 pcf; internal friction angle 34°; cohesion 0 psf. Retained earth fill/backfill: unit weight 125 pcf; internal friction angle 34°; cohesion 0 psf. Foundation soils: friction angle for sliding at base 30°, cohesion 0 psf for sliding at base. Wall foundation is anticipated over moderate bedrock or dense sandy gravel to be verified by the engineer upon the foundation excavation.

7. If actual soil, backfill or retained materials characteristics, grades or dimensions differ from those shown or depicted on these drawings, the design engineer shall be notified to re-evaluate the design of the walls.



Scatter Creek Bridge Site M.P. 3.7 - 5200 Road

| DATE March 3, 2005 | DRAWN Pacific Forest Resources Enumclaw, WA | REV |
|-----------------------|--|-------------------------|
| SCALE N/A | NE¼NE¼ Section 25, T20N, R7E, W.M., King County, Washington | ^{SHEET} 5 of 8 |

